SEOMECHANICS & GEOENGINEERING

Magued Iskander

Behavior of Pipe Piles in Sand

> Plugging and Pore Water Pressure Generating During Installation and Loading



VM Jensen

Behavior of Pipe Piles in Sand Magued Iskander,2011-01-17 One of the major difficulties in predicting the capacity of pipe piles in sand has resulted from a lack of understanding of the physical processes that control the behavior of piles during installation and loading This monograph presents a detailed blue print for developing experimental facilities necessary to identify these processes These facilities include a unique instrumented double walled pipe pile that is used to delineate the frictional stresses acting against the external and internal surfaces of the pile The pile is fitted with miniature pore pressure transducers to monitor the generation of pore water pressure during installation and loading A fast automatic laboratory pile hammer capable of representing the phenomena that occur during pile driving was also developed and used

Behavior Pf Pipe Piles in Sand Magued Iskander, 2010 Behavior of Pipe Piles in Sand Magued Iskander, 2011-03-28 One of the major difficulties in predicting the capacity of pipe piles in sand has resulted from a lack of understanding of the physical processes that control the behavior of piles during installation and loading This monograph presents a detailed blue print for developing experimental facilities necessary to identify these processes These facilities include a unique instrumented double walled pipe pile that is used to delineate the frictional stresses acting against the external and internal surfaces of the pile. The pile is fitted with miniature pore pressure transducers to monitor the generation of pore water pressure during installation and loading A fast automatic laboratory pile hammer capable of representing the phenomena that occur during pile driving was also developed and used Pore-water Pressures in Soft to Firm Clay During Driving of Piles Into Underlying Dense Sand, 1996 During driving of steel piles through soft sensitive clay into very dense sand and gravel pore water pressure responses were monitored As a result of the large length of the piles and also because of the high sensitivity of the soft clays the piles were driven in two stages During the initial stage of driving in the soft clay only very small pore water pressure increases were recorded together with very low pile driving resistances however during the second stage of driving high pore water pressure increases were observed in the clay as soon as the piles penetrated into the underlying very dense sand and gravel It was concluded that the clay deposit was loaded from below as the piles were driven into very dense sand The total stress changes and the resulting pore water pressure changes in the clay were analyzed assuming that the pile driving load was equivalent to a flexible load acting on the surface of an elastic half space which represents the soft clay deposit This interpretation of the pore water pressure increases is important for the assessment of the bearing capacity of engineering structures affected by piles driven through soft soils into very dense deposits The potential for high pore water pressure increases in the clay during undrained loading as well as for volume increases in the dense sand due to pile driving can be predicted from piezocone test data **Stress State Developments During the** Installation of Pipe Piles in Sand and Their Influence on the Lateral Load-bearing Behaviour Philipp Stein, 2023

Effect of Pile Installation Method on Pipe Pile Behavior in Sands K. Paik, R. Salgado, 2004 Open ended pipe piles are often used for the foundations of both land and offshore structures because of their relatively low driving resistance In this study calibration chamber tests were conducted on model pipe piles installed in sands with different soil conditions in order to investigate the effects of the pile installation method on penetration parameters and bearing capacity Results of the test program showed that both the hammer blow count necessary to install the piles and the incremental filling ratio IFR which is used to indicate the degree of soil plugging in open ended piles decreased 1 with increasing hammer weight for the same driving energy and 2 with increasing hammer weight at the same fall height The base and shaft load capacities of the piles were observed to increase 1 with increasing hammer weight for the same driving energy and 2 with increasing hammer weight for the same fall height It was also observed that the noise level observed during pile driving decreases 1 as the driving energy decreases and 2 as the hammer weight increases for the same driving energy Model jacked piles were also installed and tested The jacked piles were found to have higher bearing capacities than identical driven piles under similar conditions mostly due to the more effective development of soil plugging in jacking than in driving and Open-ended Pipe Piles in Sands Wissam Hadi Shijhait, Mohammed Yousif Fattah, 2014-02-28 Plugging of pipe piles is an important phenomenon which is not adequately accounted for in the current design recommendations of open ended pipe piles An open ended pipe pile is said to be plugged when the soil inside the pile moves down with the pile resulting in the pile becoming effectively closed ended Plugging is believed to result in an increase in horizontal stresses between the pile and the surrounding soil which results in an increase in skin friction The present study investigates the effect of plug on pile load capacity and effect of plug removal Different parameters are considered such as pile diameter to length ratio type of installation in sand of different densities removal of plug in three stages 50% 75% and 100% with respect to length of plug Kerbala sand which is used as a foundation soil is poorly graded clean sand The sand was prepared at different densities using a raining technique To simulate the pile load test in the field a new apparatus was manufactured A driving pressing system for pile installation was manufactured in this study. The soil plug is removed by a device manufactured to remove the soil column entrapped inside the pipe piles **Effects of Membrane Compliance on Pore Water Pressure Generation** in Gravelly Sands Under Cyclic Loading S. Mohsen Haeri, Mohammad Reza Shakeri, 2010 The paper deals with an experimental study of the undrained cyclic behavior of a natural coarse sand and gravel deposit located in Tehran a megacity situated on the continental side of the Alborz Mountain in Iran Membrane compliance that plays a significant role in inhibiting redistribution of pore pressure and liquefaction in undrained cyclic triaxial tests performed on coarse granular soils is studied in this paper Currently there is no or little satisfactory method for accounting for this phenomenon for gravelly soils and thus the non compliant cyclic loading resistance of granular soils and the evaluation of the behavior of such material in natural and in situ state are not easily determined A procedure has been proposed in this paper for consideration

of membrane compliance effects on the pore pressure measurements during cyclic loading A method is also introduced to verify the proposed procedure by employing lightly cemented specimens In addition some new correlations for excess pore water pressure ratio against the number of cycles to failure have been presented for a non compliant system under various effective confining and deviatoric cyclic stresses applied on reconstituted and isotropically consolidated specimens Initial liquefaction has been observed during the tests with or without consideration of membrane compliance however the number of cycles to initial liquefaction is higher for the compliant case **Behavior of Piles During Earthquake-induced Lateral** Pore Water Pressure Increase in Loose Saturated Sand at Level Sites During **Spreading** Privanshu Singh, 2002 Three-directional Earthquake Loading Ernesto E. Vicente, 1983 **Effect of Prefabricated Vertical Drains on Pore** Water Pressure Generation and Dissipation in Liquefiable Sand Antonio Marinucci, 2010 Soil improvement methods are used to minimize the consequences of liquefaction by changing the characteristics and or response of a liquefiable soil deposit When considering sites with previous development the options for soil improvement are limited Traditional methods such as compaction and vibratory techniques are difficult to employ because of adverse effects on adjacent structures One potential method for soil improvement against soil liquefaction in developed sites is accelerated drainage through in situ vertical drains Vertical drains expedite the dissipation of excess pore water pressures by reducing the length of the pore water drainage path For more than thirty years vertical gravel drains or stone columns have been employed to ensure the excess pore water pressure ratio remains below a prescribed maximum value In recent years the use of prefabricated vertical drains PVDs has increased because the drains can be installed with less site disruption than with traditional soil improvement methods To date little to no field or experimental verification is available regarding the seismic performance of sites treated with PVDs The effectiveness of PVDs for liquefaction remediation was evaluated via small scale centrifuge testing and full scale field testing A small scale centrifuge test was performed on an untreated soil deposit and on a soil deposit treated with small scale vertical drains Compared to the untreated condition the presence of the small scale vertical drains provided numerous benefits including smaller magnitudes of excess pore water pressure generation and buildup smaller induced cyclic shear strains reduced times for pore pressure dissipation and smaller permanent horizontal and vertical displacements In addition full scale in situ field experiments were performed in an untreated soil deposit and in a soil deposit treated with full scale PVDs using a vibrating mandrel as the dynamic source In the untreated test area the maximum induced excess pore pressure ratio reached about 0 95 In the treated test area the vibratory installation of the first few drains generated significant excess pore pressures however significant excess pore pressures were not generated during the vibratory installation of additional drains because of the presence of the adjacent drains Additionally the vibratory installation of the drains caused significant settlement and significantly altered the shear wave velocity of the sand Dynamic shaking after installation of all of the drains induced small accelerations small cyclic shear strains and negligible excess pore water

pressures in the soil The results of the field experiment indicate that the prefabricated vertical drains were effective at dissipating excess pore water pressures during shaking and densifying the site

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